

Introduction: when a fluid in motion resists other fluid properties becomes resistant, such as nature of flow of real fluid is complex.

Kinematics & hydrodynamics: - the science which deals with the geometry of motion of fluids with a reference to the forces causing motion is called as kinematics or hydrodynamics. Kinematics involves with the description of the motion of fluid in terms of space & time relations ship.

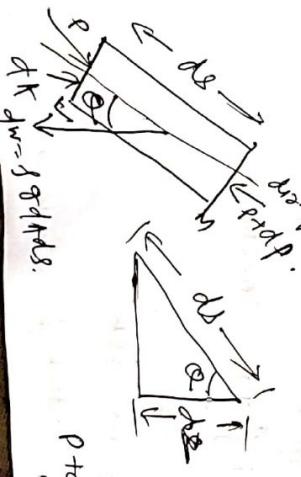
Hydrodynamics: - the science which deals with the action of forces on moving or changing motion of fluids known as hydrodynamics or simply dynamics.

The dynamics equation of fluid motion is obtained by applying Newton's 2nd law of motion to a fluid element considered as a free body. The fluid is assumed to be incompressible and non viscous.

Euler's eqn of motion

Consider steady flow of an ideal fluid along the stream tube. Taking out a small element of fluid of cross sectional area dA & length ds from stream tube as free body from the moving fluid.

The free body shows a small element M of fluid cross sectional area dA & length ds .



dA = pressure on the element
 v = velocity of the fluid elements
 ds = length of the fluid elements.

The external forces tending to accelerate the fluid element in the direction of stream line are as follows.

① Net pressure force on the direction of flow is.

$$pdA - (p + dp)dA = -dp \cdot dA. \quad \text{--- (1)}$$

② Component of the weight of the fluid element in the direction of flow

$$= -\rho g dA ds \cos \alpha$$

$$= -\rho g dA ds \left(\frac{dz}{ds} \right) \quad (\because \cos \alpha = \frac{dz}{ds}) \quad \text{--- (2)}$$

$$= -\rho g dA dz \quad \text{--- (3)}$$

mass of the fluid element $= \rho dA ds$

The acceleration of the fluid element

$$a = \frac{dv}{dt} = \frac{dN}{ds} \cdot \frac{ds}{dt} = v \cdot \frac{dN}{ds} \quad \text{--- (4)}$$

Now according to Newton's second law of motion,

Force = mass \times acceleration.

$$\therefore -dp \cdot dA - \rho g dA dz = \rho dA ds \cdot v \frac{dN}{ds}$$

Dividing both sides by ρdA , we get

$$-\frac{dp}{\rho} - g dz = v \cdot dN$$

$$\text{or } \frac{dp}{\rho} + v dv + gdz = 0 \quad \text{--- (5)}$$

This is the required Euler's equation for the motion,

and is in the form of differential equation.

Integrating the above equation, we get

$$\frac{1}{\rho} \int dp + \int v dv + \int gdz = \text{constant.}$$

$$= \frac{1}{\rho} + \frac{v^2}{2} + gz = \text{const.}$$

Dividing both sides by 'g' we get

$$\boxed{\frac{p}{\rho g} + \frac{v^2}{2g} + z = \text{constant}} \rightarrow \text{which is the Bernoulli's equation.}$$

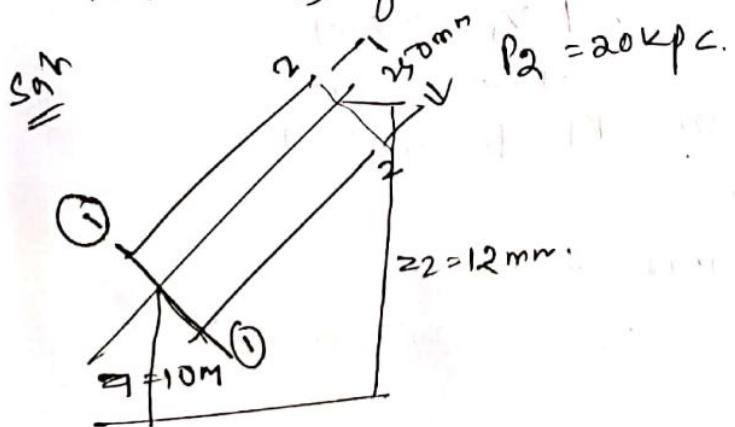
Assumptions

The following assumptions are made to derive Bernoulli's equation.

- i) The fluid is ideal & incompressible.
- ii) The flow is steady and continuous.
- iii) The flow is along the stream i.e. it is one dimensional.
- iv) The velocity is uniform over the section and is equal to the mean velocity.
- v) The only forces acting on the fluid are the gravity force and pressure force.

Numerical related to Bernoulli's eqn

Q. In a smooth inclined pipe of uniform diameter 250mm, a pressure of 50 kPa was observed at section 1 which was at elevation 10m. At another section 2 elevation 12 m, the pressure was 20 kPa and velocity was 1.25 m/s. Determine the direction of flow and the head loss between these two sections. The fluid in pipe is water. The density of water at 20°C and 760 mm Hg is 998 kg/m³.



Given

$$D = 250 \text{ mm} = 0.25 \text{ m}$$

$$P_1 = 50 \text{ kPa} = 50 \times 10^3 \text{ N/m}^2$$

$$Z_1 = 10 \text{ m}, Z_2 = 12 \text{ m}$$

$$P_2 = 20 \text{ kPa} = 20 \times 10^3 \text{ N/m}^2$$

$$\gamma = v_2 = 1.25 \text{ m/s}, \quad \delta = 998 \text{ kg/m}^3$$

Total energy at 1 + 1

$$E_1 = \frac{P_1}{\gamma g} + \frac{V_1^2}{2g} + Z_1 = \frac{50 \times 10^3}{998 \times 9.81} + \frac{1.25^2}{2 \times 9.81} + 10 = 15.187 \text{ m}$$

Total energy at 2 + 2

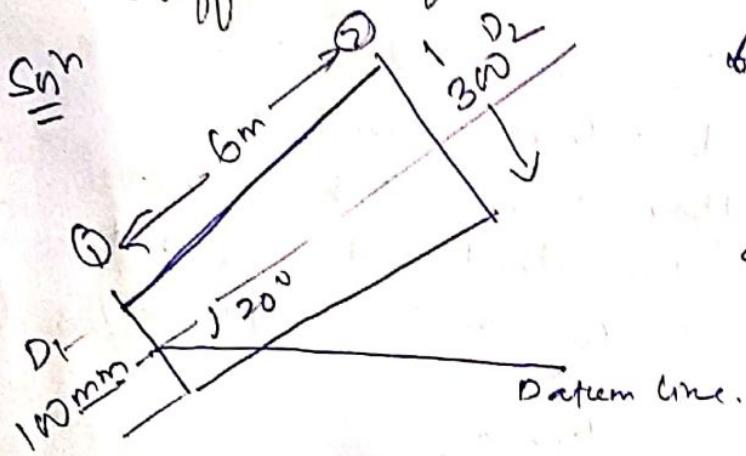
$$E_2 = \frac{P_2}{\gamma g} + \frac{V_2^2}{2g} + Z_2 = \frac{20 \times 10^3}{998 \times 9.81} + \frac{1.25^2}{2 \times 9.81} + 12 = 14.122 \text{ m}$$

$$\text{Loss of head} = h_1 = E_1 - E_2 = 19.18 - 14.12 = 1.065 \text{ m.}$$

Direction of flow

Since $E_1 > E_2$, the direction of flow from section 1-1 to 2-2.

- Q. A 6 m. long pipe is inclined at an angle of 20° with the horizontal. The smaller section of the pipe which is lower level is of 100 mm diameter and larger section of pipe is of 300 mm diameter. If the pipe is uniformly tapering and the velocity of air at the smaller section 1.8 m/s. Determine the difference of pressure between two sections.



Given Length of pipe = $l = 6\text{m}$.

- Angle of inclination = $\theta = 20^\circ$

at Secth-1

$$D_1 = 100\text{mm} = 0.1\text{m}$$

$$A_1 = \pi \frac{D_1^2}{4} = \pi (0.1)^2 = 0.00785 \text{ m}^2$$

$$\text{rel. } V_1 = 1.8 \text{ m/s}$$

$$\text{Datum } z = 0$$

$$\text{at } P_1 = \text{free air L}$$

at Secth-2

$$D_2 = 300\text{mm} = 0.3\text{m}$$

$$\text{Area } A_2 = \pi \frac{D_2^2}{4} = \pi (0.3)^2 = 0.0707 \text{ m}^2$$

$$\text{Datum } z_2 = 0 + 6 \sin 20^\circ = 6 \sin 20^\circ = 2.05 \text{ m.}$$

$$\text{Let } P_2 = \text{Pascals at Secth-2,}$$

From continuity equation, we know that

$$A_1 V_1 = A_2 V_2$$

$$V_2 = \frac{A_1 V_1}{A_2} = \frac{0.00785 \times 1.8}{0.0707} = 0.2 \text{ m/s}$$

Applying Bernoulli's eqn to both section of the pipe, we get

$$\frac{P_1}{\rho g} + \frac{V_1^2}{2g} + z_1 = \frac{P_2}{\rho g} + \frac{V_2^2}{2g} + z_2$$
$$\frac{1}{\rho g}(P_1 - P_2) = \left(\frac{V_2^2}{2g} - \frac{V_1^2}{2g} \right) + (z_2 - z_1)$$
$$= \frac{1}{2g} (0.2 m/s)^2 - 1.8^2 + (2.05 - 0) = 1.88$$

$$P_1 - P_2 = 9.81 \times 1.88 = 18.44 \text{ kN/m}^2$$

$$[\text{i.e. } W = \rho g, \text{ for water} = 9.81 \text{ kN/m}^3]$$

Practical Application of Bernoulli's equation.

Bernoulli's equation is applied to the following measuring devices.

- 1) venturi meter.
- 2) orifice meter.
- 3) pitot tube.
- 4) siphon.

Venturi meter.

It is an instrument used to measure the rate of discharge in pipeline and often fixed permanently at different sections of the pipeline to know the discharge there.

The different types of venturi meter's are

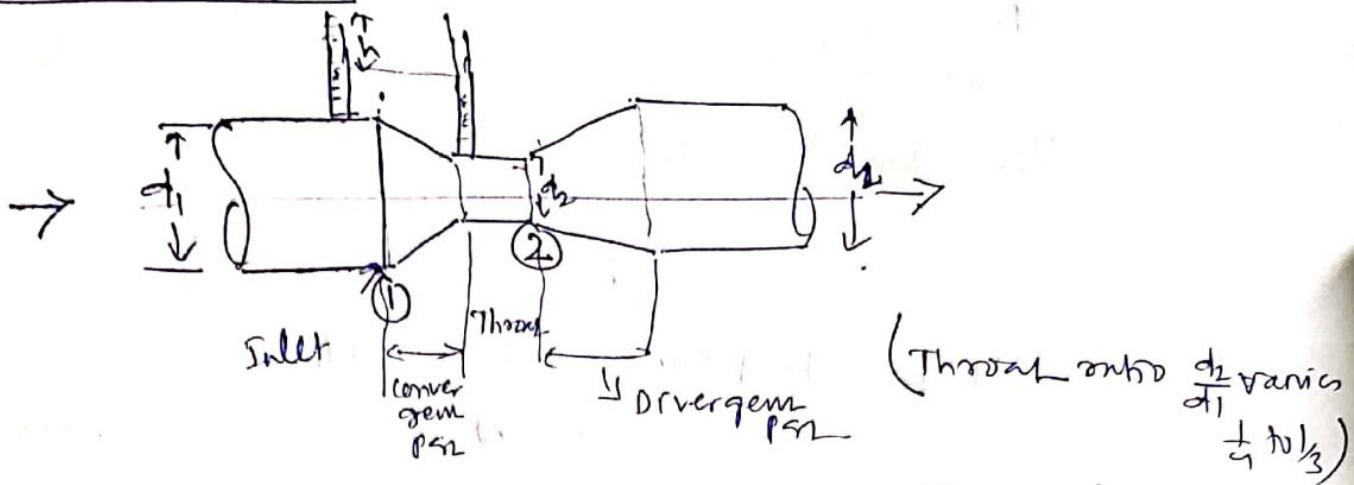
- i) horizontal venturi meter,
- ii) vertical venturi meter.

Horizontal venturi meter. A venturi meter consists of the following

three parts.

- i) A short converging part.
- ii) Throat and
- iii) Diverging part.

Expression for continuity



The above fig. shows a venturi meter fitted in a horizontal pipe through which a fluid is flowing.

$$\text{Let } D_1 = d_1 \text{ as inlet 1}$$

$$A_1 = \text{Area at inlet } (\pi d_1^2)$$

$$P_1 = \text{pressure at Sech 1}$$

$$V_1 = \text{velocity of the fluid at Sech 1}$$

and $D_2, A_2, P_2 \& V_2$ are the corresponding values at Sech -2

applying Bernoulli's equation at (1) & (2), we get

$$\frac{P_1}{\rho g} + \frac{V_1^2}{2g} + z_1 = \frac{P_2}{\rho g} + \frac{V_2^2}{2g} + z_2 \quad \text{As pipe is horizontal} \quad z_1 = z_2$$

$$\text{The equation reduces to } \frac{P_1 - P_2}{\rho g} = \frac{V_2^2}{2g} - \frac{V_1^2}{2g}. \quad \text{--- (1)}$$

But $\frac{P_1 - P_2}{\rho g}$ is difference of press head at Sech (1) & (2)

Substituting in eqn (1)

$$h = \frac{V_2^2}{2g} - \frac{V_1^2}{2g} \quad \text{--- (2)}$$

Applying Bernoulli's continuity equation. as $A_1 \& A_2$ are same

$$A_1 V_1 = A_2 V_2, \quad V_2 = \frac{A_2 V_1}{A_1}$$

Substituting the value of V_2 in eqn (2), we get

$$h = \frac{v_2^2}{2g} - \left(\frac{A_2 v_2}{A_1} \right)^2 = \frac{v_2^2}{2g} \left(1 - \frac{A_2^2}{A_1^2} \right)$$

$$\Leftrightarrow h = \frac{v_2^2}{2g} \left(\frac{A_1^2 - A_2^2}{A_1^2} \right) \Leftrightarrow v_2^2 = 2gh \left(\frac{A_1^2}{A_1^2 - A_2^2} \right)$$

$$\Leftrightarrow v_2 = \sqrt{2gh \left(\frac{A_1^2}{A_1^2 - A_2^2} \right)} = \sqrt{2gh} \sqrt{\frac{A_1}{A_1^2 - A_2^2}}$$

$$\text{Discharge } Q = A_2 v_2 = A_2 \sqrt{\frac{A_1}{A_1^2 - A_2^2}} \sqrt{2gh}$$

$$\boxed{Q = \frac{A_1 A_2}{\sqrt{A_1^2 - A_2^2}} \sqrt{2gh}} \rightarrow$$

The above equation gives the discharge under ideal conditions is called theoretical discharge;

Actual discharge =

$$Q_{act} = C_d \frac{A_1 A_2}{\sqrt{A_1^2 - A_2^2}} \sqrt{2gh}$$

where C_d = co-efficient of discharge of venturiometer.
and the value is less than unity. (varies between 0.96 and 0.98)

* Due to variation of C_d venturiometers are not suitable for very low velocities.

value of 'h' by differential manometer.

case-1 - differential manometer containing a liquid heavier than the liquid flowing through the pipe.

then

$$h = Y \left[\frac{S_{he} - 1}{S_p} - 1 \right]$$

where S_{he} = sp. gravity of heavier liquid.

S_p = sp. gravity of liquid flowing through pipe.

Y = Difference of the heavier liquid column in U-tube.

Case - 2 - Differential manometer containing a liquid lighter than the liquid flowing through the pipe.

$$h = \gamma \left(1 - \frac{S_e}{S_p} \right)$$

where

S_e = Sp. gravity of lighter liquid.
 S_p = Sp. gravity of liquid flowing through pipe.

γ = Difference of lighter liquid.

Problem related to venturi meter

A horizontal venturi meter with inlet diameter 200 mm & throat diameter 100 mm is used to measure the flow of water. The pressure at inlet is 0.18 N/mm^2 and the vacuum pressure at the throat is 280 mm of Hg. Find the rate of flow. Take $C_d = 0.98$.

Soln Given

$$\text{Inlet dia} = D_1 = 200 \text{ mm} = 0.2 \text{ m.}$$

$$A_1 = \pi \frac{(0.2)^2}{4} = 0.0314 \text{ m}^2$$

$$\text{Diameter at Throat} D_2 = 100 \text{ mm} = 0.1 \text{ m}$$

$$A_2 = \pi \frac{(0.1)^2}{4} = 0.00785 \text{ m}^2$$

$$\text{Pressure at inlet } P_1 = 0.18 \text{ N/mm}^2 = 180 \text{ kN/m}^2$$

$$\frac{P_1}{\rho g} = \frac{180}{9.81} = 18.3 \text{ m.}$$

Vacuum pressure at throat

$$\begin{aligned} \frac{P_2}{\rho g} &= -280 \text{ mm of Hg.} = -0.28 \text{ m of Hg} \\ &= -0.28 \times 13.6 = -3.8 \text{ m of water.} \end{aligned}$$

coefficient of discharge $C_d = 0.98$

$$\text{Differential head} = h = \frac{P_1}{\rho g} - \frac{P_2}{\rho g} = 18.3 - (-3.8) = 22.1 \text{ m}$$

$$\text{Hence Rate of flow} = Q = C_d \times \frac{A_1 A_2}{\sqrt{A_1^2 - A_2^2}} \sqrt{2gh} = 0.98 \times \frac{0.0314 \times 0.00785}{\sqrt{(0.0314)^2 - (0.00785)^2}} \times 22.1$$

$$Q = 0.165 \text{ m}^3/\text{s}$$

Q-2 - A horizontal venturi meter with inlet & throat diameters 300mm & 100mm respectively is used to measure the flow of water. The pressure intensity at inlet is 130 kN/m^2 while the vacuum pressure head at the throat is 350mm of mercury. Assuming that 3 percent of head is lost between the inlet & throat, find.

- (i) The value of C_d (co-efficient of discharge) for the venturi meter.
- (ii) Ratio of flow.

Sol Given - Inlet dia $D_1 = 300 \text{ mm} = 0.3 \text{ m}$, $A_1 = \pi \times 0.3^2 = 0.07 \text{ m}^2$
 Throat dia $D_2 = 100 \text{ mm} = 0.1 \text{ m}$, $A_2 = \pi \times 0.1^2 = 0.00785 \text{ m}^2$
 Pressure at inlet $P_1 = 130 \text{ kN/m}^2$
 Pressure head $\frac{P_1}{\rho g} = \frac{130}{9.81} = 13.25 \text{ m}$.

Similarly, pressure head at throat $= \frac{P_2}{\rho g} = -350 \text{ mm of mercury}$
 $= -0.35 \times 13.6 \text{ m of water}$
 $= -4.76 \text{ m}$.

- (i) co-efficient of discharge C_d .

Differential head, $h = \frac{P_1}{\rho g} - \frac{P_2}{\rho g} = 13.25 - (-4.76) = 18.01 \text{ m}$.

Head lost $h_f = 3\% \text{ of } h = \frac{3}{100} \times 18.01 = 0.54 \text{ m}$.

Hence $C_d = \sqrt{\frac{h - h_f}{h}} = \sqrt{\frac{18.01 - 0.54}{18.01}} = 0.985$.

- (ii) Ratio of discharge Q

$$Q = C_d \times \frac{A_1 A_2}{\sqrt{A_1^2 - A_2^2}} \times \sqrt{2gh}$$

$$= 0.985 \times \frac{0.07 \times 0.00785}{\sqrt{0.07^2 - 0.00785^2}} \times \sqrt{2 \times 9.81 \times 18.01}$$

$$= \frac{0.000541}{0.0956} \times 18.79 = 0.146 \text{ m}^3/\text{s}$$

Vertical and inclined venturiometers

vertical & inclined venturiometers are employed for measuring discharge on pipeline which are not horizontal.

$$\text{Here } h = \left(\frac{P_1}{\gamma_g} - \frac{P_2}{\gamma_g} \right) + (z_1 - z_2)$$

Q. A vertical venturiometer carries a liquid of relative density 0.8 & has inlet and throat diameters of 150mm & 75mm respectively. The pressure connection at throat is 150mm above that at the inlet. If the actual rate of flow is 40 lit/sec and $C_d = 0.96$, calculate the pressure difference between inlet & throat in N/m²

Sol: Given - Sp. grath = 0.8

$$D_1 = 150\text{mm} = 0.15\text{m}, \quad A_1 = \pi \frac{D_1^2}{4} = \pi \frac{(0.15)^2}{4} = 0.01767\text{m}^2$$

$$D_2 = 75\text{mm} = 0.075\text{m}, \quad A_2 = \pi \frac{D_2^2}{4} = \pi \frac{(0.075)^2}{4} = 0.00442\text{m}^2$$

$$z_1 - z_2 = 150\text{mm} = 0.15\text{m}$$

$$Q_{act} = 40 \text{lit/sec} = \frac{40}{1000} = 0.04 \text{m}^3/\text{s}, C_d = 0.96$$

$$Q_{act} = C_d \frac{A_1 A_2}{\sqrt{A_1^2 - A_2^2}} \times \sqrt{2gh}$$

$$0.04 = 0.96 \times \frac{0.01767 \times 0.00442}{\sqrt{(0.01767)^2 - (0.00442)^2}} \sqrt{2 \times 9.81 \times h}$$

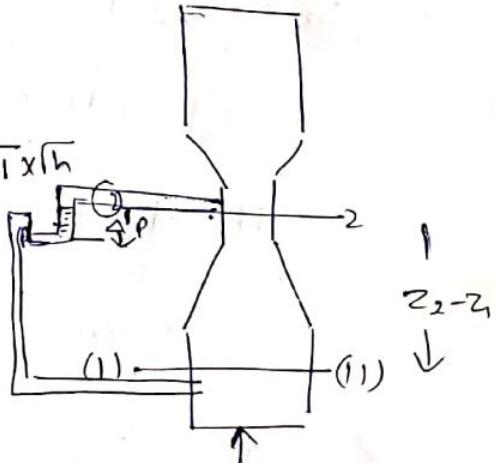
$$\text{Solving } h = 4.247 \text{m.}$$

$$\text{As we know } h = \left(\frac{P_1}{\gamma_g} - \frac{P_2}{\gamma_g} \right) + (z_1 - z_2)$$

$$2.247 = \left(\frac{P_1 - P_2}{\gamma_g} \right) - 0.15$$

$$P_1 - P_2 = \gamma_g (2.247 + 0.15) = (0.8 \times 1000 \times 9.81) (4.247 + 0.15)$$

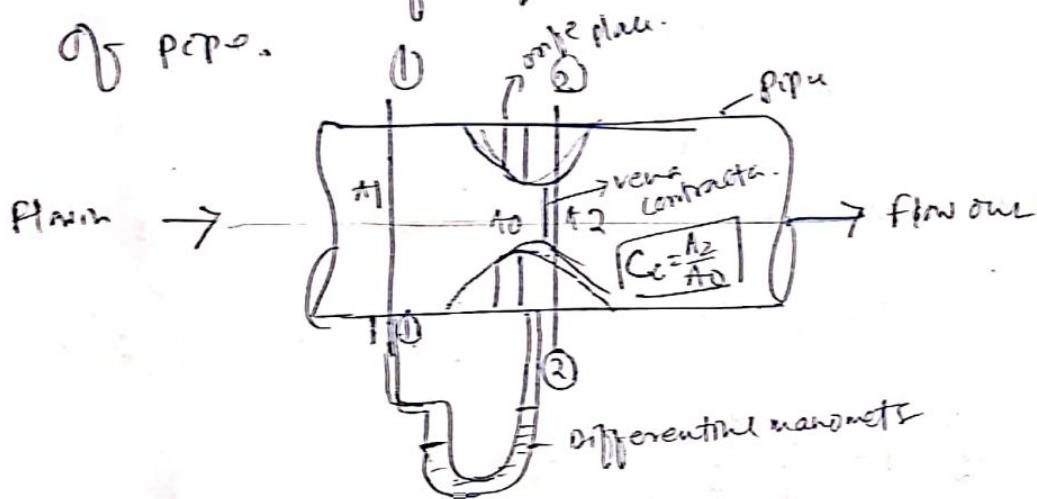
$$= 34.51 \text{ kN/m}^2$$



Orrizometer - 1

Orrizometer is a device employed for measuring the discharge of fluid through a pipe. It also works on the same principle of a venturi meter.

It consists of a flat circular plate having a circular sharp edged hole (called orifice) concentric with pipe. The dia of orifice is 0.4 to 0.8 time the diameter of pipe.



Let A_1 = Area of pipe at Section 1.

v_1 = velocity at Section 1.

P_1 = pressure at Section 1, and

A_2, v_2, P_2 are corresponding values at Section 2.

Applying Bernoulli's equation at (1) & (2)

$$\frac{P_1}{\rho g} + \frac{v_1^2}{2g} + z_1 = \frac{P_2}{\rho g} + \frac{v_2^2}{2g} + z_2$$

$$\left(\frac{P_1}{\rho g} + z_1 \right) - \left(\frac{P_2}{\rho g} + z_2 \right) = \frac{v_2^2}{2g} - \frac{v_1^2}{2g}$$

$$\therefore h = \frac{v_2^2}{2g} - \frac{v_1^2}{2g}, \text{ where } h = \left(\frac{P_1}{\rho g} + z_1 \right) - \left(\frac{P_2}{\rho g} + z_2 \right) = \text{differential pressure head}$$

$$\frac{v_2^2}{2g} = h + \frac{v_1^2}{2g} \quad \text{--- (1)}$$

$$\therefore v_2 = \sqrt{2gh + \frac{v_1^2}{2g}} = \sqrt{2gh + v_1^2}$$

at section 2) is at venae contractae. A_2 represents the area of venae contractae. If A_0 is the area of orifice then

$$C_c = \frac{A_2}{A_0} \quad , \text{ where } C_c = \text{coefficient of contraction.}$$

$$A_2 = A_0 C_c \quad \text{--- (2)}$$

Using continuity equation, we get,

$$A_1 V_1 = A_2 V_2$$

$$V_1 = \frac{A_2 V_2}{A_1} = \frac{A_0 C_c V_2}{A_1} \quad \text{--- (3)}$$

Substituting the value of V_1 in equation '1', we get.

$$V_2 = \sqrt{2gh + \frac{A_0^2 C_c^2 V_2^2}{A_1^2}} = 2gh \left(\frac{A_0}{A_1}\right)^2 C_c^2 V_2^2$$

$$\text{or } V_2^2 \left[1 - \left(\frac{A_0}{A_1}\right)^2 C_c^2 \right] = 2gh$$

$$\therefore V_2 = \frac{\sqrt{2gh}}{1 - \left(\frac{A_0}{A_1}\right)^2 C_c^2}$$

Again the discharge, $Q = A_2 V_2 = A_0 C_c V_2$

$$= A_0 C_c \frac{\sqrt{2gh}}{\sqrt{1 - \left(\frac{A_0}{A_1}\right)^2 C_c^2}} \quad \text{--- (4)}$$

The above expression is simplified by writing

$$C_d = C_c \frac{\sqrt{1 - \left(\frac{A_0}{A_1}\right)^2}}{\sqrt{1 - \left(\frac{A_0}{A_1}\right)^2 C_c^2}}$$

where C_d = coefficient of discharge.

$$\text{or } C_c = C_d \frac{\sqrt{1 - \left(\frac{A_0}{A_1}\right)^2 C_d^2}}{\sqrt{1 - \left(\frac{A_0}{A_1}\right)^2}}$$

Substituting the value of C_e in equation (4), we get

$$Q = A_0 C_d \frac{\sqrt{1 - (A_0/A_1)^2} C_e^2}{\sqrt{1 - (A_0/A_1)^2}} \times \frac{\sqrt{2gh}}{\sqrt{1 - (A_0/A_1)^2} C_e^2}$$

$$= C_d A_0 \sqrt{2gh} = \frac{C_d \cdot A_0 A_1 \sqrt{2gh}}{\sqrt{A_1^2 - A_0^2}}$$

i.e.

$$Q = C_d \frac{A_0 A_1 \sqrt{2gh}}{\sqrt{A_1^2 - A_0^2}}$$

The coefficient of discharge of orifice meters is much smaller than that of venturi meters.

Difference between venturi meters & orifice meters

Venturi meters

- i) venturi meters can be used for measuring the flow rate of all incompressible flows. (gases with low pressure variation as well as liquids)
- ii) venturi meters are used for pipelines only and the accelerated flow through the apparatus is subsequently decelerated to original velocity at the outlet of the venturi meter.
- iii) The flow velocity measured by noting the pressure difference between the inlet and throat of the venturi meter.

Orifice meters

- i. & ii) used for measuring flow rate of liquids.
- ii) In orifice meter the entire potential energy of the fluid is converted into KE, & the jet discharges freely into the open atmosphere.
- iii) The discharge velocity is measured by using pitot tube or by trajectory method.

Q. For an orifice meter, diameter of the pipe = 240 mm.
 Diameter of the orifice = 120 mm.
 Specific gravity of oil = 0.88
 Reading of differential manometer = 40 mm of oil.
 Coefficient of discharge = 0.64.

Determine the ratio of flow.

Sol

$$D_1 = 240 \text{ mm} = 0.24 \text{ m}, \quad A_1 = \frac{(0.24)^2}{4} = 0.0452 \text{ m}^2$$

$$D_0 = 120 \text{ mm} = 0.12 \text{ m} \Rightarrow A_2 = \frac{\pi}{4} (0.12)^2 = 0.0113 \text{ m}^2$$

$$\text{Differential head: } h = \left(\frac{h_1}{S_o} - l \right) = 0.4 \left[\frac{13.6}{0.88} - 1 \right] \\ = 5.78 \text{ m. of oil.}$$

$$\text{Discharge } Q = C_d \frac{A_1 A_0 \sqrt{2gh}}{\sqrt{A_1^2 - A_0^2}}$$

$$= 0.64 \times \frac{0.0113 \times 0.452 \times \sqrt{2 \times 9.81 \times 5.78}}{(0.0452)^2 - (0.0113)^2}$$

$$= \frac{0.000353}{0.0137} = 0.026 \text{ m}^3/\text{s.}$$